

**PROPERTY OF OSJ BERLIN LLC
FOR ENTERPRISE HOLDINGS
127 WEBSTER SQUARE ROAD
BERLIN, CONNECTICUT**

DRAINAGE CALCULATIONS

JULY 2022

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DRAINAGE CALCULATIONS

JULY 2022

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I. Drainage Statement

Project Location

The proposed project is located in the Webster Square Plaza on the west side of Webster Square Road, in the town of Berlin, CT.

Scope of Project

The existing site is a 15 acre retail plaza. The project site is approximately 0.4 acre consisting of an office space in the existing building and renovations to an exterior portion of the building once occupied by a part of the existing building damaged by a fire and since razed. The renovations consist of removing a portion of the existing weathered and deteriorating concrete floor slab and portions of the old foundation wall, regrading the area and paving with new bituminous concrete to create a parking area for the rental vehicles. A small storm drainage system is being added to the site to capture and treat the storm water prior to it entering the existing storm drainage system. Treatment will be by a PIG ® oil and sediment catch basin insert that fits into the new catch basin frame.

Drainage Analysis and Design Methodology

The proposed storm sewer was sized utilizing the Rational Method and NOAA Atlas 14 data for the 10-year storm event and checked against the 25-year storm event and found to be adequate for the 25-year storm. The storm sewer pipe system design calculations and inlet data are included in this report. The analysis utilized the Hydraflow Storm Sewers 2005 and Hydraflow Hydrographs 2004 from IntelliSolve. The software performs analysis consistent with ConnDOT requirements.

II. REDEVELOPMENT HYDROGRAPH

Computer Software Utilized:

Hydraflow Hydrographs Program 2004

By: InteliSolve

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Hydrograph Return Period Recap

Hyd. No.	Hydrograph type (origin)	Inflow Hyd(s)	Peak Outflow (cfs)								Hydrograph description
			1-Yr	2-Yr	3-Yr	5-Yr	10-Yr	25-Yr	50-Yr	100-Yr	
1	Rational	-----	-----	2.288	-----	2.956	3.518	4.298	4.889	5.462	Enterprise
Proj. file: Enterprise site.gpw									Wednesday, Jun 15 2022, 11:54 AM		

4

Hydrograph Summary Report

Hyd. No.	Hydrograph type (origin)	Peak flow (cfs)	Time interval (min)	Time to peak (min)	Volume (cuft)	Inflow hyd(s)	Maximum elevation (ft)	Maximum storage (cuft)	Hydrograph description
1	Rational	3.518	1	5	1,055	----	-----	-----	Enterprise
Enterprise site.gpw					Return Period: 10 Year			Wednesday, Jun 15 2022, 11:54 AM	

Hydrograph Summary Report

Hyd. No.	Hydrograph type (origin)	Peak flow (cfs)	Time interval (min)	Time to peak (min)	Volume (cuft)	Inflow hyd(s)	Maximum elevation (ft)	Maximum storage (cuft)	Hydrograph description
1	Rational	4.298	1	5	1,290	----	-----	-----	Enterprise
Enterprise site.gpw					Return Period: 25 Year			Wednesday, Jun 15 2022, 11:54 AM	

III. Storm Sewer System Calculations

Computer Software Utilized:

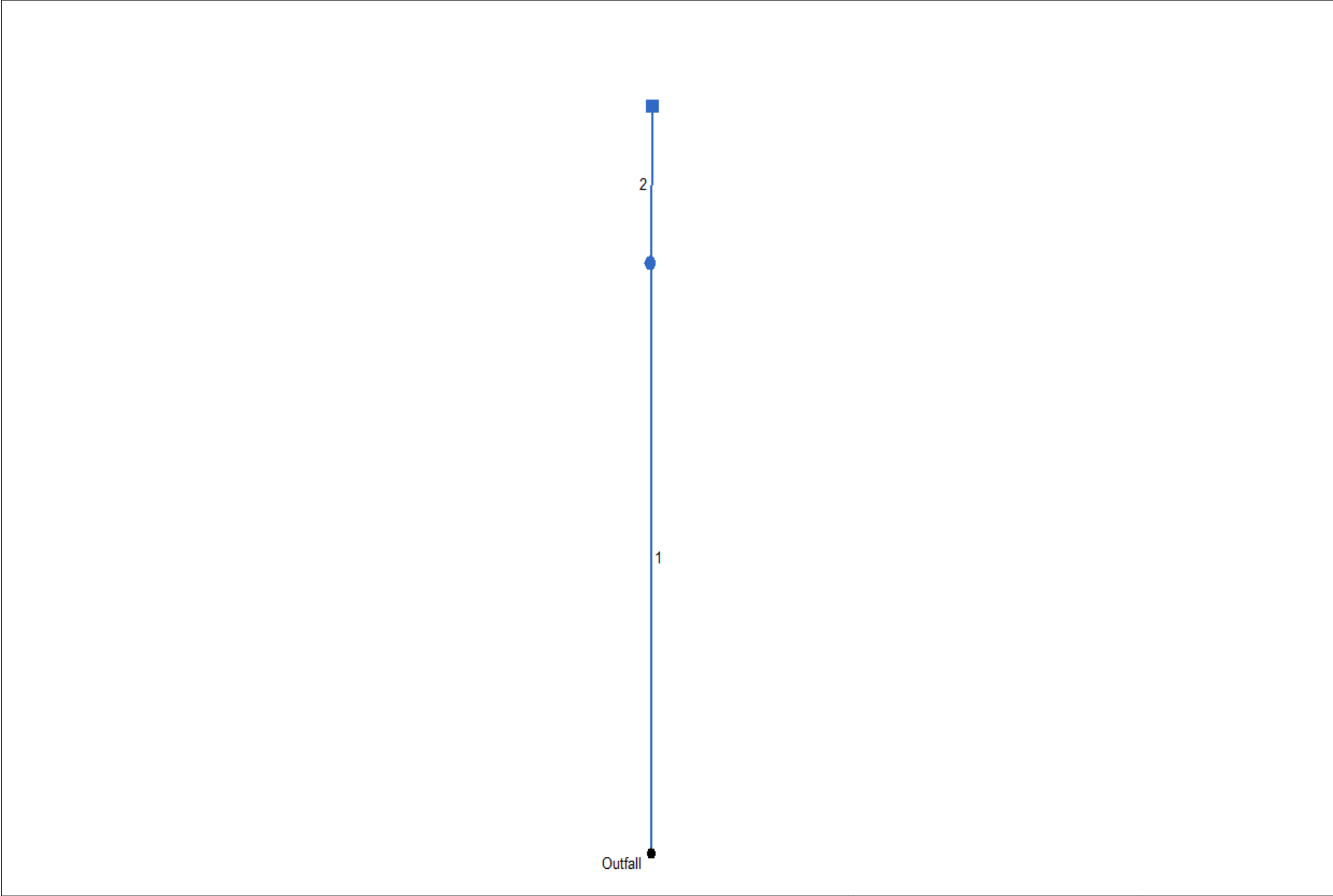
Hydra Flow Storm Sewers Program 2005

By: InteliSolve

Design Criteria:

- Rational Method
- NOAA Atlas 14 - 10-and 25 Year Storm Events

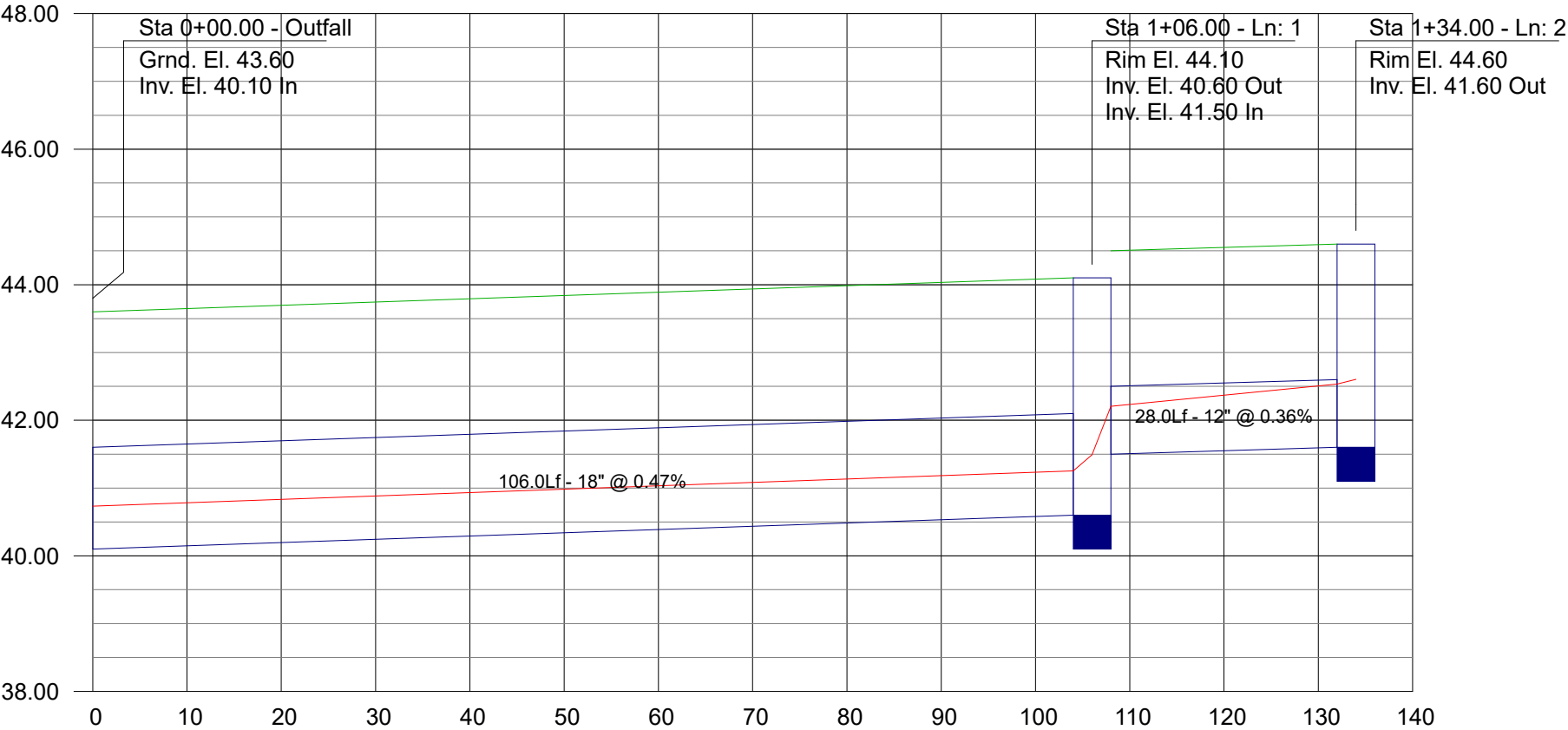
Hydraflow Plan View



Enterprise - Webster Sq	No. Lines: 2	07-14-2022
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Storm Sewer Profile

Elev. (ft)



Hydraulic Grade Line Computations

Line	Size	Q	Downstream								Len	Upstream								Check		JL coeff	Minor loss
			Invert elev (ft)	HGL elev (ft)	Depth (ft)	Area (sqft)	Vel (ft/s)	Vel head (ft)	EGL elev (ft)	Sf (%)		Invert elev (ft)	HGL elev (ft)	Depth (ft)	Area (sqft)	Vel (ft/s)	Vel head (ft)	EGL elev (ft)	Sf (%)	Ave Sf (%)	Enrgy loss (ft)		
(1)	(2)	(3)	(4)	(5)	(6)	(7)	(8)	(9)	(10)	(11)	(12)	(13)	(14)	(15)	(16)	(17)	(18)	(19)	(20)	(21)	(22)	(23)	(24)
1	18	2.76	40.10	40.73	0.63	0.71	3.88	0.23	40.97	n/a	106	40.60	41.25	0.65	0.74	3.74	0.22	41.47i	n/a	n/a	0.284	0.15	n/a
2	12	2.78	41.50	42.21	0.71*	0.59	4.68	0.34	42.55	n/a	28.0	41.60	42.53	0.93	0.76	3.64	0.21	42.74i	n/a	n/a	-0.015	1.00	n/a
Enterprise - Webster Sq														Number of lines: 2					Run Date: 07-14-2022				
Notes: * Critical depth assumed.																							

General Procedure: Hydraflow computes the HGL using the Bernoulli energy equation. Manning's equation is used to determine energy losses due to pipe friction. In a standard step, iterative procedure, Hydraflow assumes upstream HGLs until the energy equation balances. If the energy equation cannot balance, supercritical flow exists and critical depth is temporarily assumed at the upstream end. A supercritical flow Profile is then computed using the same procedure in a downstream direction using momentum principles. The computed HGL is checked against inlet control.

Col. 1 The line number being computed. Calculations begin at Line 1 and proceed upstream.

Col. 2 The line size. In the case of non-circular pipes, the line rise is printed above the span.

Col. 3 Total flow rate in the line.

Col. 4 The elevation of the downstream invert.

Col. 5 Elevation of the hydraulic grade line at the downstream end. This is computed as the upstream HGL + Minor loss of this line's downstream line.

Col. 6 The downstream depth of flow inside the pipe (HGL - Invert elevation) but not greater than the line size.

Col. 7 Cross-sectional area of the flow at the downstream end.

Col. 8 The velocity of the flow at the downstream end, (Col. 3 / Col. 7).

Col. 9 Velocity head (Velocity squared / 2g).

Col. 10 The elevation of the energy grade line at the downstream end, HGL + Velocity head, (Col. 5 + Col. 9).

Col. 11 The friction slope at the downstream end (the S or Slope term in Manning's equation).

Col. 12 The line length.

Col. 13 The elevation of the upstream invert.

Col. 14 Elevation of the hydraulic grade line at the upstream end.

Col. 15 The upstream depth of flow inside the pipe (HGL - Invert elevation) but not greater than the line size.

Col. 16 Cross-sectional area of the flow at the upstream end.

Col. 17 The velocity of the flow at the upstream end, (Col. 3 / Col. 16).

Col. 18 Velocity head (Velocity squared / 2g).

Col. 19 The elevation of the energy grade line at the upstream end, HGL + Velocity head, (Col. 14 + Col. 18) .

Col. 20 The friction slope at the upstream end (the S or Slope term in Manning's equation).

Col. 21 The average of the downstream and upstream friction slopes.

Col. 22 Energy loss. Average Sf/100 x Line Length (Col. 21/100 x Col. 12). Equals (EGL upstream - EGL downstream) +/- tolerance.

Col. 23 The junction loss coefficient (K).

Col. 24 Minor loss. (Col. 23 x Col. 18). Is added to upstream HGL and used as the starting HGL for the next upstream line(s).

IV. Inlet Data

Drainage Areas: 20,680 s.f.

CB2 Area Total	20,680 s.f.	Cn 0.90
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CB1 Area Total	n/a s.f.	modelled as a manhole due to its location on a side slope, most runoff will by pass and be captured in CB 2
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VI. Stormwater Quality

A water quality flow was calculated according to the 2004 Connecticut Stormwater Quality Manual for the proposed site. That flow is 0.5 c.f.s. The proposed site storm runoff will be captured in two catch basins that will have an oil and sediment filters install that will handle flows as high as 600 gallons per minute, far in excess of what is expected here.

Proposed Water Quality Flow

WQV = Water Quality Volume (ac-ft.)

R = Volumetric Runoff Coefficient = $0.05 + 0.009 I$

I = Percent Impervious Cover = 90%

A = Site Area in Acres = 0.47 acres

I = 90 R = $0.05 + 0.009 (90) = 0.86$

WQV = $\frac{(1") (0.86) (0.47)}{12} = 0.034 \text{ ac-ft.}$

WQF A = 0.00074 mi²

Q = $\frac{\text{WQV} \times 12 \text{ in./ft.}}{\text{Drainage Area (acres)}} = \frac{0.034 \text{ ac-ft.} (12)}{0.47 \text{ ac}} = 0.86 \text{ in.}$

CN = $\frac{1000}{(10 + 5P + 10Q - 10 [Q^2 + 1.25 (QP)]^{1/2})} =$

$\frac{1000}{(10 + 5 + 10 (0.86) - 10 [(0.86)^2 + 1.25 (0.86)]^{1/2}} = 98.7$

CN = 99 Ia = 0.041 Ia/p = 0.041 Qu = 650

WQF = (qu) (A) (Q) = 650 (0.00074) (0.86) = 0.41 c.f.s.

Total WQF from Site, use 0.5 c.f.s.

APPENDIX

